GEOTECHNICAL REPORT for the HAINES SMALL BOAT HARBOR HAINES, ALASKA

May 2001

1. Scope

The results of a reconnaissance study and test pit exploration for the proposed small boat harbor at Haines, Alaska are presented in this report.

The scope of the investigation was to:

1) gain a general understanding of the subsurface conditions within exploration limits with regard to dredging, 2) aid in developing criteria for planning a detailed subsurface exploration, and 3) provide preliminary recommendations relevant to the design and construction of the proposed small boat harbor.

2. Project Location and Description

Haines is located in southeast Alaska about 121 kilometers (km) northwest of Juneau at the head of Lynn Canal. The project area is located on the waterfront bordering the shore and adjacent to each side of the existing city harbor. Two areas are under consideration for the project. The area to the south of the city harbor is herein designated Portage Cove South. The proposed work at that location would consist of extending the existing seaward breakwater southward and constructing a new breakwater from shore to provide enclosure. The other area, designated Portage Cove North, would involve constructing a new harbor north of the existing city harbor. Harbor dredging and construction of breakwaters will be the primary components of the project. Proposed dredging will extend to elevations between -4.3 to -5.5 meters (m) MLLW. For reference, a Project Location Map is enclosed.

3. Field Explorations

The current exploration was conducted on 9 September 2000. Fourteen test pits were excavated; thirteen (TP-2002 through TP-2014) to the north of the existing harbor and one (TP-2001) to the south. The test pits were excavated to depths from 1.25 m to 4 m below ground surface with a tracked CAT 307 backhoe

fitted with an 0.46 m bucket. Mr. Mike Murphy of Southeast Road Builders operated the backhoe. The excavation of the test pits was supervised and logged by a geologist with the Corps of Engineers in conformance with ASTM D2488-93, "Standard Practice for Description and Identification of Soils (Visual - Manual Procedure)." Grab samples of the primary material types encountered in the test pits were obtained for laboratory testing to help in soils classification. The test pits north of the harbor were located using a cloth tape measuring perpendicularly from stationing on a previously established That baseline originated at the cable crossing sign on the north edge of the project area and extended along the high water line. Control was also provided by reference to the intersections of Front Street with Union and Dalton Streets. The test pit south of the harbor was located using the fuel storage facility and the walkway to the dock as references. The locations are shown on the enclosed Test Pit Location Map.

4. Laboratory Testing and Soil Classification

A testing program using the test methods listed below was established to determine the physical properties of selected soil samples. A Corps of Engineers approved laboratory performed the testing.

ASTM D 422-63 (Re-approved 1990), "Standard Test Method for Particle Size Analysis of Soils".

ASTM D 2216-92, "Standard Test Method for Laboratory Determination of Water (Moisture) Content of Soil and Rock".

ASTM D 2487-93, "Standard Test Method for Classification of Soils for Engineering Purposes (Uniform Soil Classification System)".

ASTM D 4318-95a, "Standard Test Method for Liquid Limit, Plastic Limit, and Plasticity Index for Soils".

TM 5-822-5/AFM 88-7 Chapter 1, "Pavement Design for Roads, Streets, Walks, and Open Storage Areas", for determining the frost classification of the soil.

The soil descriptions and classifications contained in this report and presented on the final exploration logs are the project geologist's interpretation of the project field logs and

results of laboratory testing. The stratification lines represent approximate boundaries between soil types; actual transitions are often gradual. The test pit logs and gradation curves are enclosed.

5. Site Conditions

Geologic Setting:

Haines is located on a peninsula dividing two inlets branching from a navigable fjord called the Lynn Canal that is collocated with the Chatam Straight Fault. This fault is a strand of the Denali fault where underwater and the Chilkat fault where it extends onto land. In the Haines area, the fault separates on the west a package of metasedimentary, metavolcanic and metaplutonic rocks which have been subjected to low grade metamorphism from rocks to the east affected by high temperature and pressure events associated with the emplacement of the coast range. In the Haines area, activity has been noted on this fault although elsewhere it is considered inactive.

Southeastern Alaska lies in one of the two most seismically active zones in Alaska, a state where 6 percent of the world's shallow earthquakes have been recorded. Between 1899 and 1970, five earthquakes having magnitudes of 8 or greater have occurred in or near southeastern Alaska or in adjacent offshore areas (Lemke, 1975). In November 1987, an earthquake measuring 5.3 on the Richter scale had an epicenter at Haines. Haines lies within Seismic Zone III. It is possible that if the area is strongly shaken by an earthquake, that the harbor facilities and other man made structures in low-lying parts of the city may be the most heavily damaged. Non-engineered loose fills placed along the shore to elevate low-lying areas are expected to be subject to comparatively strong shaking. These areas are also subject to settlement, possible liquefaction, water-sediment ejection, ground fracturing, and landsliding.

Surface:

The tidal fluctuation at Haines ranges approximately from MLLW of 0.0 m and MHHW of 5.12 m on U.S. Coast and Geodetic Survey records. The proposed harbor sites are located within this intertidal zone and the subtidal zone. No bedrock was observed at the project sites.

Portage Cove South

One test pit, TP-2001 was excavated in this area. Access was difficult because of obstacles posed by a field of boulders with dimensions up to $1.5\ \mathrm{m}$.

Portage Cove North

Thirteen test pits were excavated in this area. The surface is covered with silty sand, gravel, cobbles, boulders and marine vegetation. Many boulders have dimensions up to 1.5 m. The boulders are subrounded to subangular, hard, metamorphic and plutonic.

In the grassy upland area, a clay unit intersects the surface at an elevation of about $1.5\ \mathrm{m}.$

Subsurface:

General

Within the intertidal zone of the areas of interest, the primary soil types encountered during the exploration or reported during dredging the city harbor are 1) diamicton, 2) interlayered silts and sands, 3) silty sand with gravel, cobbles and boulders, and 4) lean clay which locally contains gravel, cobbles and/or boulders. Diamicton is composed of poorly sorted or unsorted sediments that consist of particles from sand to boulder sizes in a matrix of fine sand, silt and/or clay. consolidated, typically has the appearance of concrete, and could be likened to an aggregate filled, hard mudstone. hardness is attributed to compression by Pleistocene glaciers. A second soil type consists of alternate layers of compacted silt and sand with minor gravel lenses that was reported during harbor dredging. A silty sand with gravel, cobbles, boulders and marine vegetation generally mantles the project areas and ranges in thickness from 0.15 m to about 1 m. A lean clay was encountered north of the existing harbor and is gray, plastic, and relatively soft to stiff.

Portage Cove South

The surfacial silty sand with gravel was encountered in TP-2001 to a depth of 1.2 m. Diamicton was encountered below the sand and became increasingly hard until refusal at 1.3 m below ground surface. The diamicton is composed of about 30% gravel, 50% sand and 20% fines.

Portage Cove North

The surfacial silty sands with gravel were typically encountered to a depth of 0.2 m on the north side of the city harbor. Lean clay with a sand fraction of less than 5% was encountered below the surfacial sand to the limit of the test pits. This clay is massive except for some thin beds of sand and shells in test pits TP-2007 and TP-2008. Boulders were embedded in the clay in five of the test pits.

6. Discussion and Analysis

Several circumstances pose a risk for the proposed construction of the breakwater. These include:

- 1) the possible presence of undiscovered bedrock,
- 2) the possible extended presence of diamicton at the south site,
- 3) the possible settlement of breakwater structures,
- 4) the stability of entrance channel and harbor basin side slopes.

These items are discussed below:

Bedrock: The precipitous nature of the upland topography at Haines suggests that underwater topography may be similarly irregular and that the potential in the project area for submerged rock exists. Previous work in the area of the current harbor did not encounter bedrock. The possible presence of bedrock is always a major concern during dredging operations. Its removal requires expensive underwater drilling and blasting. It would be advisable to conduct a seismic survey to confirm the absence of bedrock, the density of and distribution of sediments, as well as to delineate the occurrence(s) of diamicton. The occurrence of identified bedrock and the need to minimize its effect on project costs could influence the selection of the site and the geometry of the harbor.

 $\underline{\text{Diamicton}}$: The presence of diamicton encountered in TP-2001 is another dredging concern. Its removal is not as expensive as bedrock, but it is reported to be about 2 to 3 times that of conventional dredging. Dredging of diamicton is difficult due to its hard, dense nature and due to possibly

embedded cobbles and boulders. It is often considered dredgeble with appropriate equipment (ie, large backhoes), but slower progress or even a need to blast should be expected. As with bedrock presence, the harbor basin and entrance channel should be positioned to minimize quantities of diamicton requiring removal. Again, the dredging quantities must be accurate and sufficient geotechnical data concerning the nature of material to be removed is imperative.

Settlement: Based on field observations recorded in the central and northern project area (the Portage Cove North proposal), consideration should be given to a settlement study. Lean clay is the dominant material underlying the project area north of the existing harbor. In general, this clay is massive and extends below the limit of this exploration. (This clay only partially occurred at the current harbor site. For example, Felix Toner described a strata or pocket of firm clay in the northeast corner of the current harbor, and indicates that to the south the soils consist of interlayered silt and sand).

Stability: The side slopes of the entrance channel and harbor basin will be influenced by tidal fluctuation and wave erosion as well as soil type. These conditions should be taken into account when selecting slope angles. Another consideration is structures constructed near the top of the side slopes. These structures should be offset an adequate distance to insure stability.

7. Recommendations

The construction of a small boat harbor at the sites can be accomplished. However, several conditions and construction aspects should be carefully assessed. It is recommended that a detailed geotechnical exploration be performed once the final harbor site has been selected. The exploration should focus on identifying the boundaries between the shore deposits, diamicton, and bedrock, if present. It is recommended that the exploration include geophysical techniques as well as test borings and test pits. An undisturbed sample of the lean clay should be obtained from the proposed harbor site for characterization. Side slopes considered appropriate for design are 3H:1V for the shore deposits, 2H:1V for the diamicton, and 1H:4V for bedrock. For preliminary design purposes, it is recommended that structures be offset a minimum of 3.0 m from the top of dredged side slopes.

Enclosures:

- 1. Vicinity Map
- 2. Test Pit Location Map
- 3. Test Pit Logs (TP-2001 through TP-2014)
- 4. Grain-Size Distribution Curves
- 5. Photographs

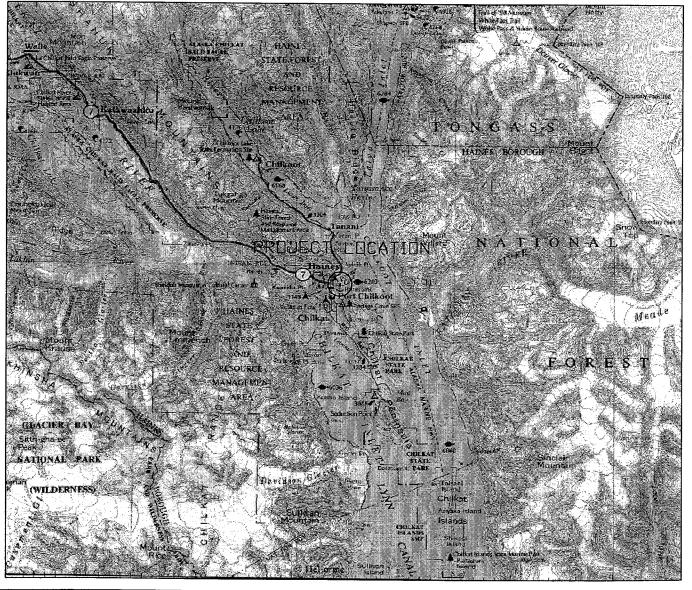
REFERENCES:

75-250, "Reconnaissance Engineering Geology of the Ketchikan Area, Alaska, with Special Emphasis on Evaluation of Earthquake and Other Geologic Hazards".

, George Plafker, and Kirk Dixon, 1980, Geological Survey Circular 844, "Horizontal Offset History of the Chatham Strait Fault".

., 1957, "Subsurface Investigation, Small Boat Harbor Site Haines, Alaska". This work also reports the results of Project Aaa. 50-4-290 directed by Daniel Cole.





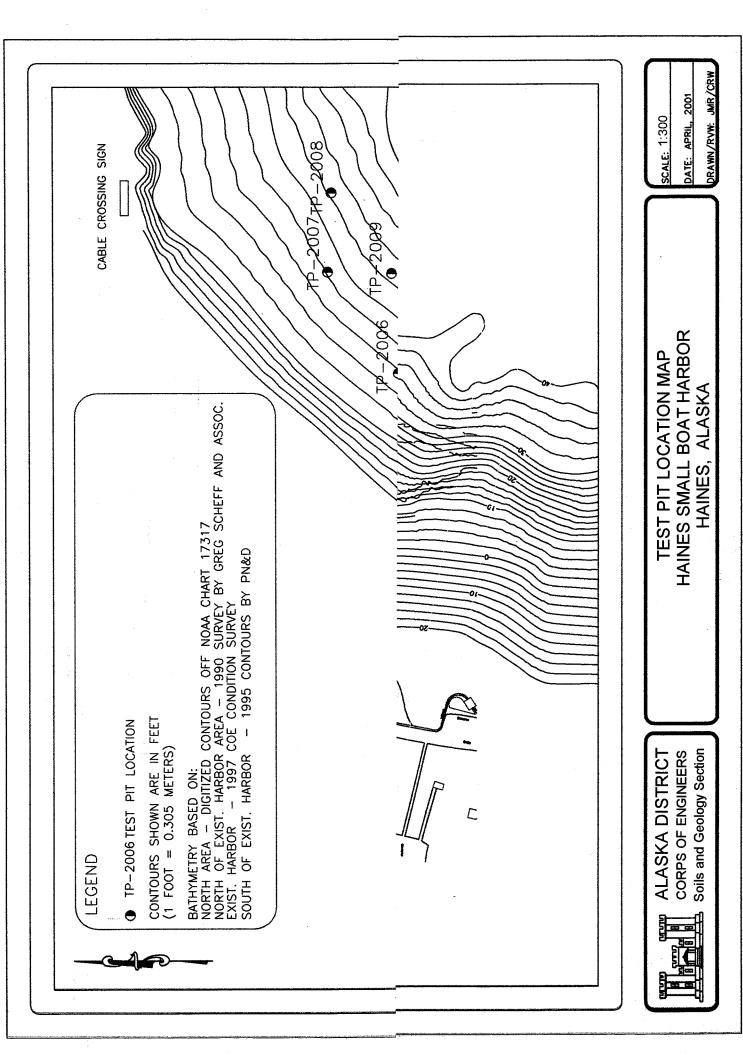


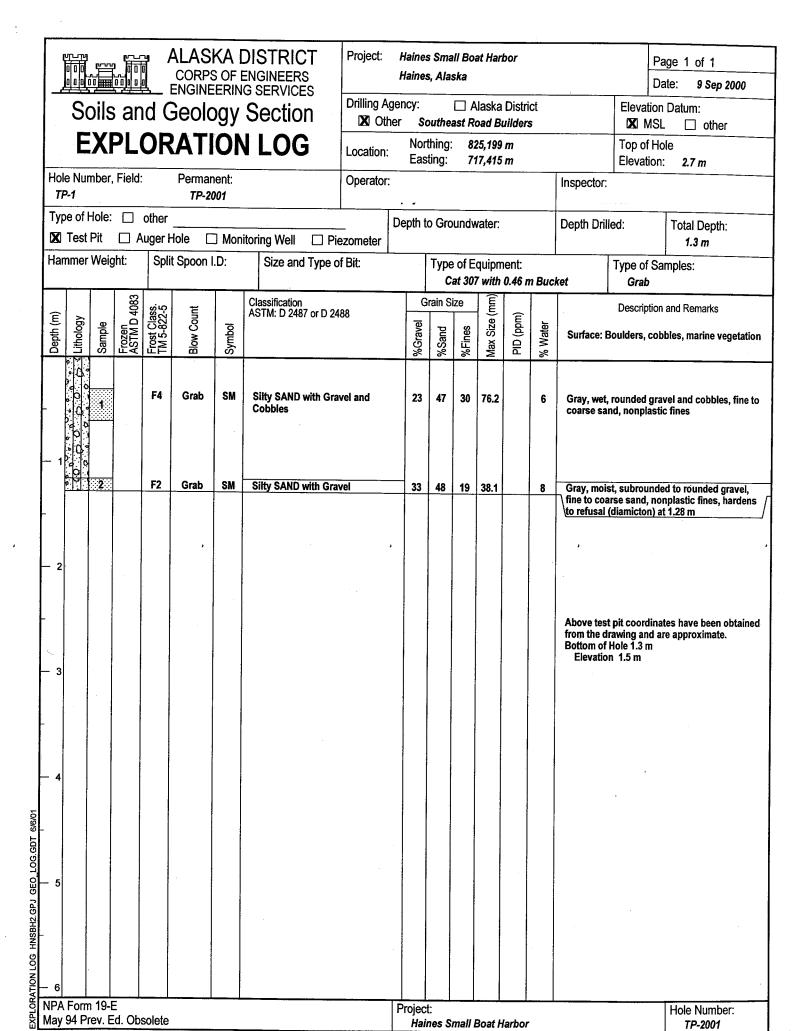
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HAINES SMALL BOAT HARBOR
HAINES, ALASKA

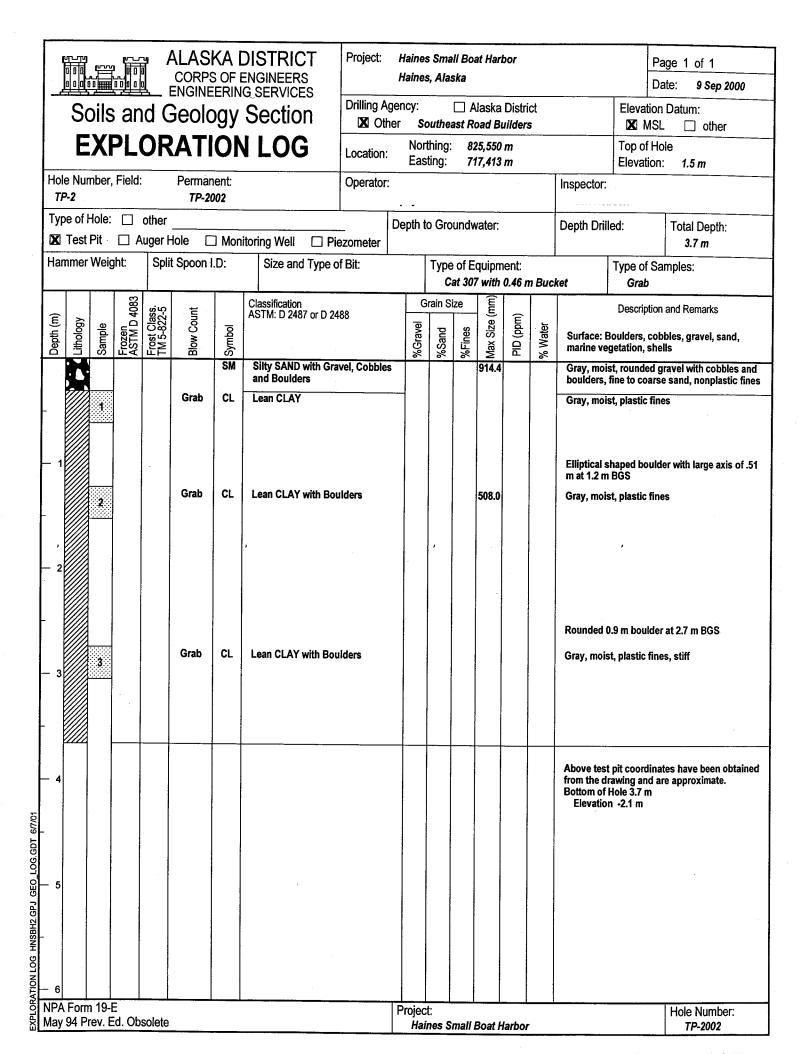
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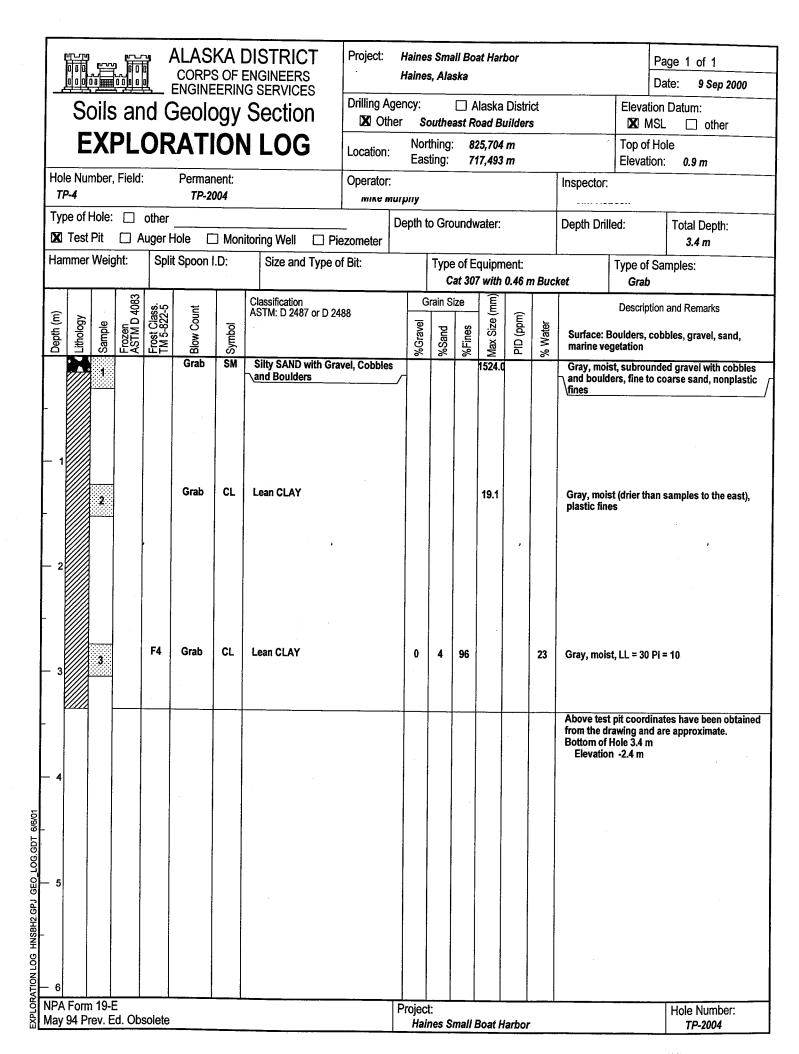
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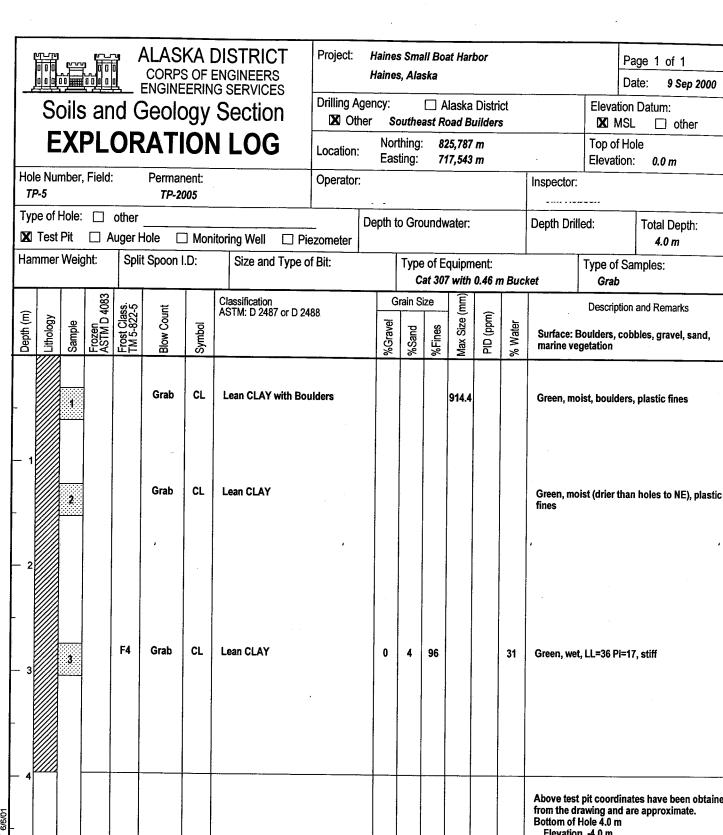






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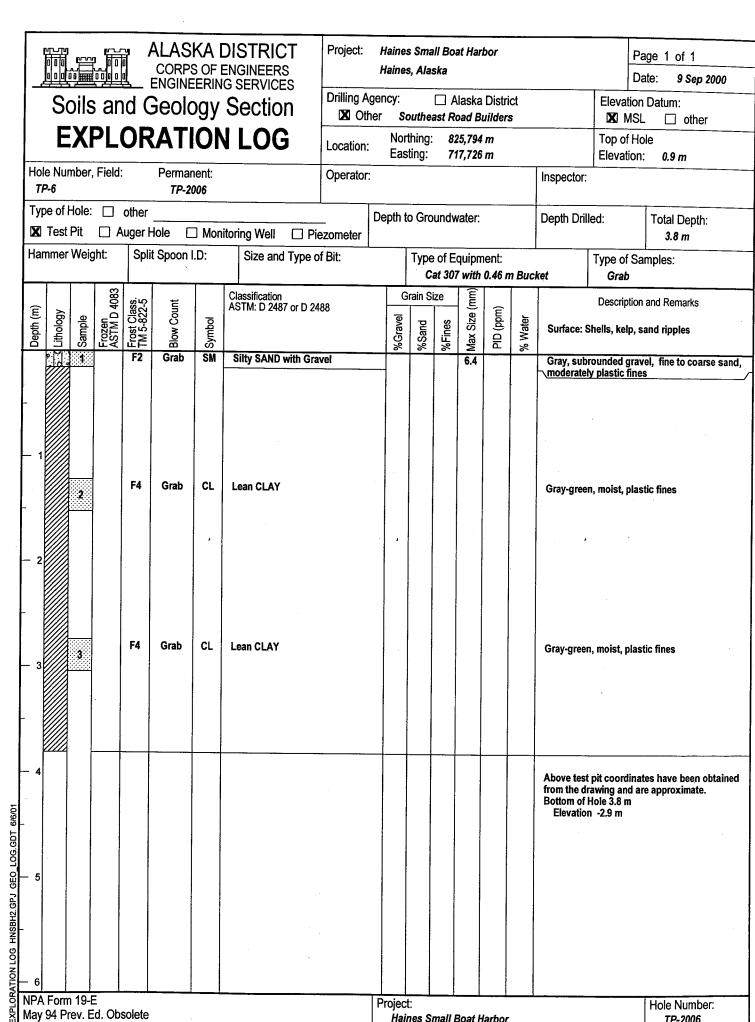
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Haines Small Boat Harbor

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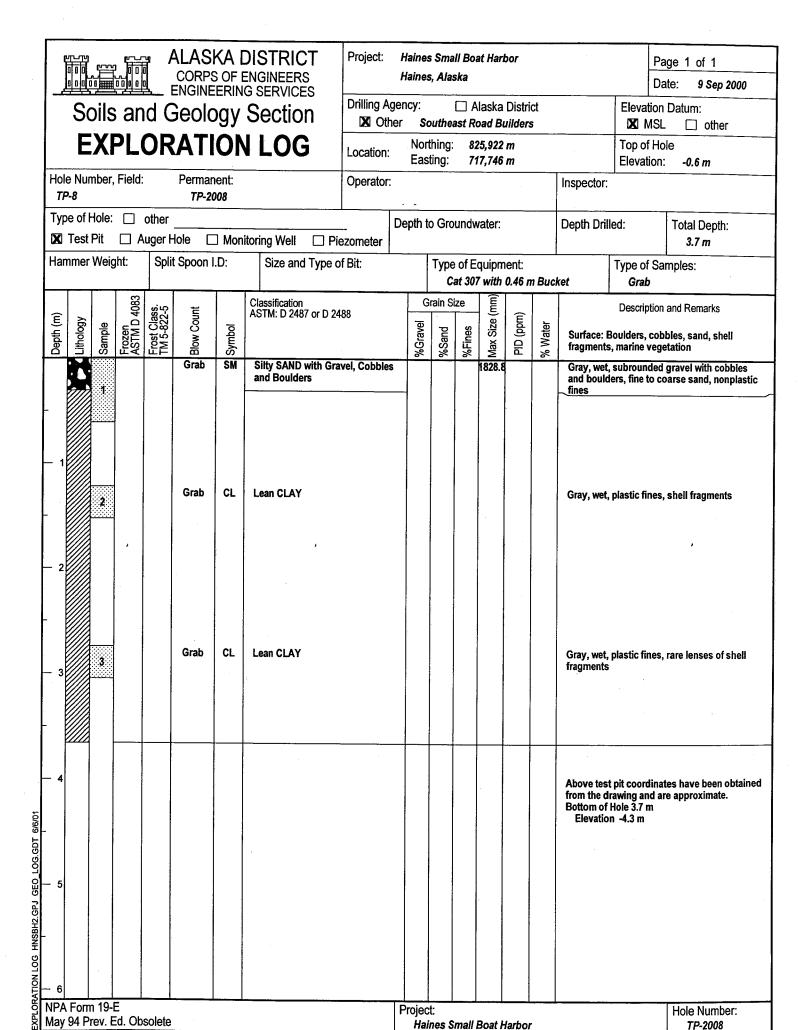


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	e Nur 2-13	nber,	Field	:	Permai			Operator	r:		·····				Inspector:	<u> </u>	
	e of I Test			other Auger		☐ Mon	itoring Well	– ezometer	Depth	to Gr	ound	water:			Depth Drill	ed:	Total Depth:
Han	nmer	Weig			it Spoon	I.D:	Size and Type o	f Bit:				quipm 7 with		n Buc	ket	Type of S Grab	Samples:
(m	3y .		Frozen ASTM D 4083	lass. 22-5	ount		Classification ASTM: D 2487 or D 24	88	-	Grain (Т	e (mm)	ê			Descripti	on and Remarks
Depth (m)	Lithology	Sample	Frozen ASTM I	Frost Class. TM 5-822-5		Symbol			%Gravel	%Sand	%Fines	Max Size (mm)	PID (ppm)	% Water	Surface: I vegetation	Boulders, g 1	gravel, sand, marine
		1			Grab	SM	Silty SAND with Gra and Boulders	vel, Cobble	es			1219.2			Gray, mois	ers, fine to	nded gravel with cobbles coarse sand, moderately
- 1		2		F4	Grab	CL	Lean CLAY		4	10	86	19.1	•	23			.07 m BGS ne to coarse sand, LL = 27 ,
- 3		3			Grab	CL	Lean CLAY								Gray-greer	ı, moist, pla	astic fines
- 4															Above test from the di Bottom of I Elevation	awing and Hole 3.4 m	nates have been obtained are approximate.
- 5	T PP																
- 6																	
		19-E					· · · · · · · · · · · · · · · · · · ·		Projec	L	l						

			<u> </u>	CORP: ENGINE	S OF I	DISTRICT ENGINEERS G SERVICES	Project:	Haines Haines			at Hari	bor				Page 1 of 1 Date: 9 Sep 2000
						Section	Drilling A	• •	uthe		Alaska Road B				Elevation XI M	on Datum: SL
E	X	PL	OR	RAT	01	1 LOG	Location	. Norl Eas			25,497 17,404				Top of I	
Hole Nu	mber,	, Field	:	Permai			Operator	:						Inspector:		
Type of Test					 ∃ Mon	itoring Well	ezometer	Depth to	Gro	ound	water:	٠		Depth Dril	lled:	Total Depth:
Hammer	Weig	ght:	Spli	it Spoon	I.D:	Size and Type of	of Bit:				quipm 7 with		n Buc	ket	Type of S	Samples:
(E) 25		2 4083	lass. 22-5	onut		Classification ASTM: D 2487 or D 24	188	-	rain S	T	e (mm)	Œ			Descripti	on and Remarks
Depth (m) Lithology	Sample	Frozen ASTM D 4083	Frost Class. TM 5-822-5	Blow Count	Symbol			%Gravel	%Sand	%Fines	Max Size (mm)	PID (ppm)	% Water	Surface: I debris	Boulders, c	obbles, gravel, wood
	1			Grab	SM	Silty SAND with Gra and Boulders	vel, Cobble	S			914.4			Gray, moi and bould plastic fin	ders, fine to	ded gravel with cobbl coarse sand, moderat
- 1	2		F4	Grab , Grab	CL	Lean CLAY with Bot	,		5	95	457.2		19	Boulder to	o 0.46 m at 2	s, plastic fines 2.7 m BGS ded gravel, LL=29 PI=
5														from the d Bottom of	rawing and	nates have been obtain are approximate.
6 IPA Form								Project	·						·	Hole Number:
lay 94 P	rev. E	d. Ob	solete							mall	Boat H	arbor				TP-2014

Client: U. S. Army Corps of Engineers

Project: Haines Small Boat Harbor

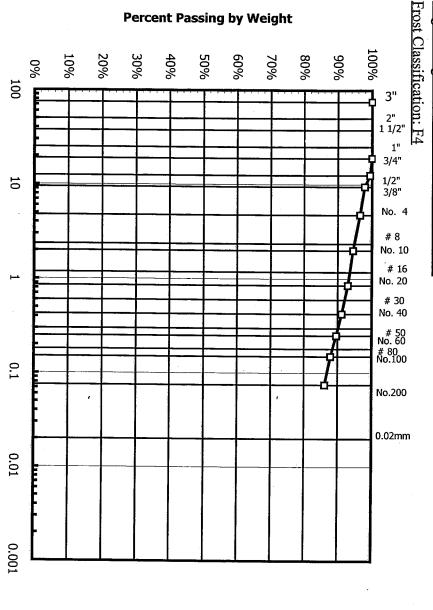
10-60

Location: Pit No. 13, SA-2 @-9-0'-10-0'

Submitted by Client

LL = 27, PI = 9, Moisture Content = 23.4%

Engineering Classification: Lean CLAY, CL



© Alaska Testlab, 1999

Particle Size (mm)

0.02 mm

otal Wt. of Fine Fraction = 0g

88% 86%

No. 100

No. 60

90%

No. 30 No. 40 No. 50

91%

No. 16 No. 20

93%

PARTICLE-SIZE

W.O. A29088

Lab No. 2191

Received: 10/25/00

PASSING SPECIFICATION
+3 in Not Included in Test = ~0%
100%
99%
98%
96%
Total Wt. = 727.6g
94%

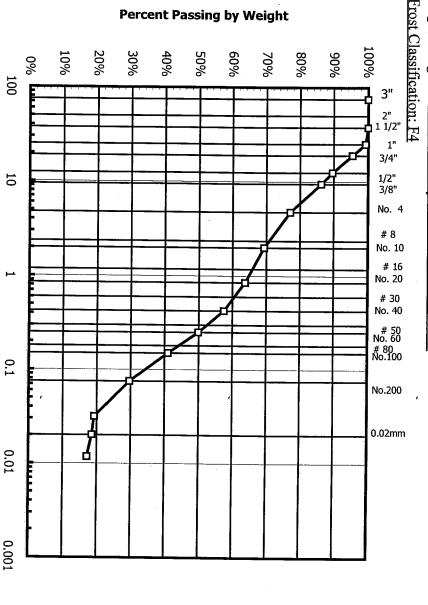
Client: U. S. Army Corps of Engineers

Location: Pit No. 1, SA-1 @ 1.0'-2.0'

Submitted by Client

PI = Non Plastic, Moisture Content = 6.3%

Engineering Classification: Silty SAND with Gravel, SM



© Alaska Testlab, 1999

Particle Size (mm)

0.02 mm

18.5%

otal Wt. of Fine Fraction = 0g

41% 30%

No. 80 No. 100

No. 40 No. 50 No. 60

50%

57%

No. 16 No. 20 No. 30

64%

PARTICLE-SIZE

DIST. ASTM D422

W.O. A29088 Lab No. 2187

Received: 10/25/00

SIZE	PASSING SPECIFICATION
+3 in Not It	+3 in Not Included in Test = ~0%
3"	
2"	
1 1/2"	100%
1"	99%
3/4"	95%
1/2"	90%
3/8"	86%
No. 4	77%
Total Wt. = 3591.9g	3591.9g
No. 8	
No. 10	69%

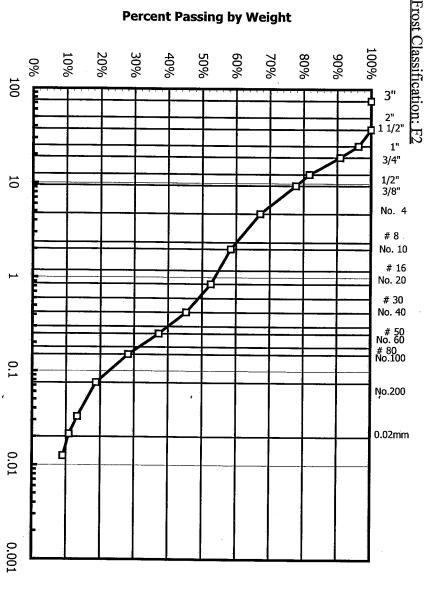
Client: U. S. Army Corps of Engineers
Project: Haines Small Boat Harbor

Location: Pit No. 1, SA-2 @ 3.8'-4.2'

Submitted by Client

PI - Non Plastic, Moisture Content = 7.8%

Engineering Classification: Silty SAND with Gravel, SM



© Alaska Testlab, 1999

Particle Size (mm)

0.02 mm

10.7%

PARTICLE-SIZE

DIST. ASTM D422 W.O. A29088

Lab No. 2186

Received: 10/25/00

SIZE	PASSING SPECIFICATION
+3 in Not Inc	S.
ယ္ခ	
2"	
1 1/2"	100%
1:	96%
3/4"	91%
1/2"	82%
3/8"	78%
No. 4	67%
Total Wt. = 1	1487.2g
No. 8	
No. 10	58%
No. 16	
No. 20	52%
No. 30	
No. 40	45%
No. 50	
No. 60	37%
No. 80	
No. 100	28%
No. 200	19%
Total Wt. of F	Total Wt. of Fine Fraction = 0g
2	

Client: U. S. Army Corps of Engineers

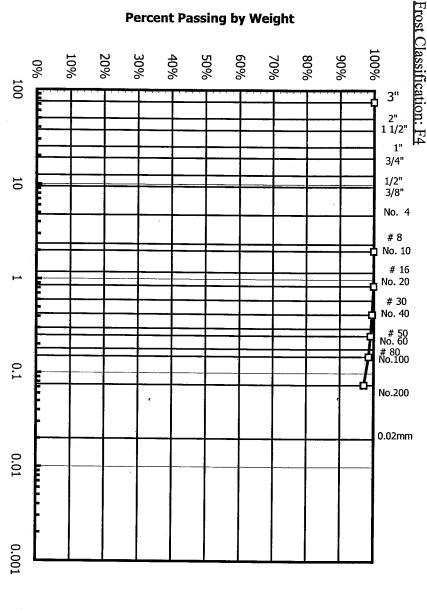
Project: Haines Small Boat Harbor

Location: Pit No. 10, SA-2 @ 4.0'-5.0'

Submitted by Client

LL = 31, PI = 12, Moisture Content = 29.9%

Engineering Classification: Lean CLAY, CL



© Alaska Testlab, 1999

Particle Size (mm)

PARTICLE-SIZE

DIST. ASTM D422 W.O. A29088

Lab No. 2192

Received: 10/25/00

	SIZE P	PASSING SPECIFICATION
	+3 in Not Inclu	S.
	3.	
	2"	
	1 1/2"	
	7	
	3/4"	
	1/2"	
	3/8"	
г	No. 4	
L	Total Wt. = 202.8g	.8g
	No. 8	
	No. 10	100%
	No. 16	
- توسر	No. 20	100%
	No. 30	
<u> </u>	No. 40	99%
17	No. 50	
-	No. 60	99%
17	No. 80	
17	No. 100	98%
l-y	No. 200	97%
П	Total Wt. of Fine Fraction = 0g	Fraction = 0g
_	0.02 mm	

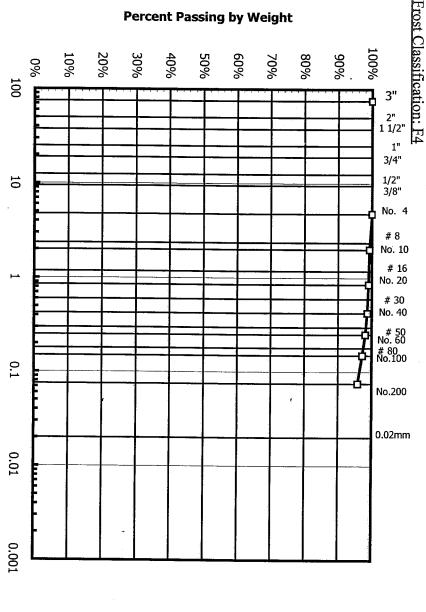
Client: U. S. Army Corps of Engineers

Location: Pit No. 4, SA-3 @ 9.0'-10.0'

Submitted by Client

LL = 30, PI = 10, Moisture Content = 23.2%

Engineering Classification: Lean CLAY, CL



© Alaska Testlab, 1999

Particle Size (mm)

0.02 mm

[otal Wt. of Fine Fraction = 0g

96% 97%

No. 100

No. 80 No. 60 No. 50 No. 40

98%

99%

PARTICLE-SIZE

DIST. ASTM D422

Lab No. 2190 W.O. A29088

Received: 10/25/00

SIZE PASSING SPECIFICATION
+3 in Not Included in Test = ~0%
3"
2"
1 1/2"
1"
3/4"
1/2"
3/8"
No. 4 100%
Total Wt. = 237.6g
No. 8
No. 10 99%
No. 16
No. 20 99%
No. 30

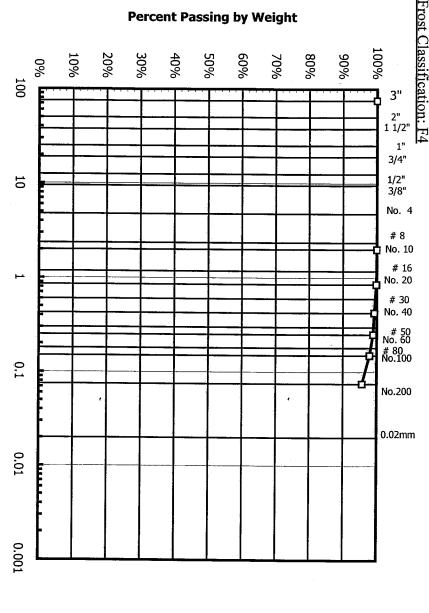
Client: U. S. Army Corps of Engineers

Location: Pit No. 5, SA-3 @ 9.0'-10.0'

Submitted by Client

LL = 36, PI = 17, Moisture Content = 30.6%

Engineering Classification: Lean CLAY, CL



© Alaska Testlab, 1999

Particle Size (mm)

0.02 mm

PARTICLE-SIZE

DIST. ASTM D422

W.O. A29088

Lab No. 2188

Received: 10/25/00

SIZE	PASSING SPECIFICATION
+3 in Not In	in Not Included in Test = ~0%
3"	
2"	
1 1/2"	
1"	
3/4"	
1/2"	
3/8"	
No. 4	
Total Wt. = I	118.8g
No. 8	
No. 10	100%
No. 16	
No. 20	100%
No. 30	
No. 40	99%
No. 50	
No. 60	99%
No. 80	
No. 100	98%
No. 200	96%
Total Wt. of I	Total Wt. of Fine Fraction = 0g

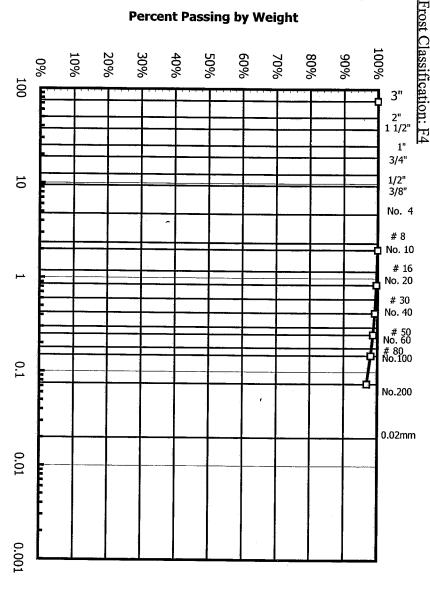
Client: U. S. Army Corps of Engineers

Location: Pit No. 12, SA-3 @ 9.0'-10.0'

Submitted by Client

LL = 30, PI = 11, Moisture Content = 24.5%

Engineering Classification: Lean CLAY, CL



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Particle Size (mm)

PARTICLE-SIZE

DIST. ASTM D422

W.O. A29088 Lab No. 2189

Received: 10/25/00

SIZE	PASSING SPECIFICATION
+3 in Not Incl	in Not Included in Test = ~0%
3=	
2"	
1 1/2"	
1	
3/4"	
1/2"	
3/8"	
No. 4	
Total Wt. = 26	263g
No. 8	
No. 10	100%
No. 16	
No. 20	100%
No. 30	
No. 40	99%
No. 50	
No. 60	99%
No. 80	
No. 100	98%
No. 200	97%
Total Wt. of Fine	ne Fraction = 0g
$0.02 \mathrm{mm}$	-

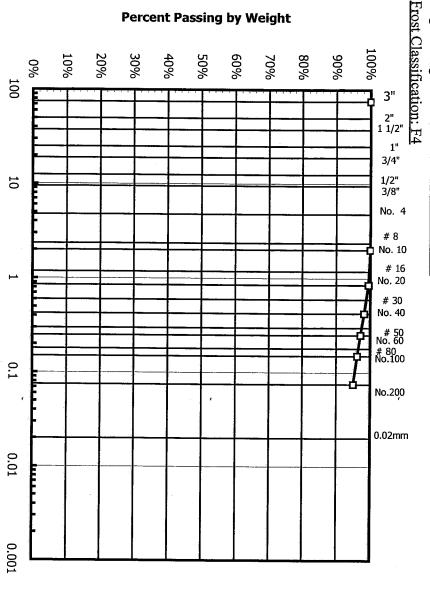
Client: U. S. Army Corps of Engineers
Project: Haines Small Boat Harbor

Location: Pit No. 14, SA-3 @ 11.0'-12.0'

Submitted by Client

LL = 29, PI = 10, Moisture Content = 18.6%

Engineering Classification: Lean CLAY, CL



© Alaska Testlab, 1999

Particle Size (mm)

0.02 mm

PARTICLE-SIZE

W.O. A29088

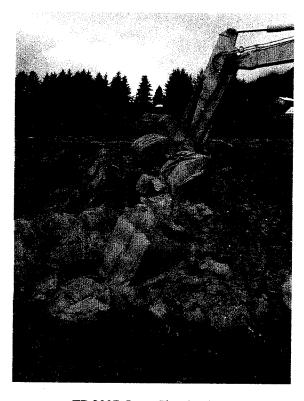
Lab No. 2193

Received: 10/25/00

Total Wt. of Fine Fraction = 0g	No. 200 9	No. 100 9	No. 80	No. 60 9	No. 50	No. 40	No. 30	No. 20	No. 16	No. 10 10	No. 8	Total Wt. = $187.9g$	No. 4	3/8"	1/2"	3/4"	1:	1 1/2"	2"	သူ	+3 in Not Included in Test =	SIZE PASSING
action = 0g	95%	96%		97%		98%		99%		100%				,						·	in Test = ~0%	ING SPECIFICATION



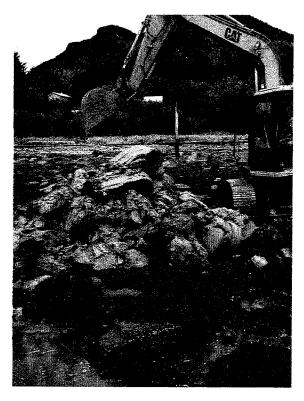
TP-2006: Lean Clay Cuttings



TP-2007: Lean Clay Cuttings



TP-2012 Note Boulders Behind Lean Clay Cuttings



TP-2007: Boulders Overlying Lean Clay

HAINES SMALL BOAT HARBOR IDEALIZED BREAKWATER STABILITY ANALYSIS

Introduction

The Corps of Engineers' Alaska District Hydraulic and Hydrology (H&H) section (CEPOA-EN-CW-HH) requested an idealized stability analysis for a breakwater to be constructed on clay soils at the Haines Small Boat Harbor. The results of the analysis are to be used for project planning, determining costs, and viability. This report is in response to H&H's request.

Proposed Design

Two idealized breakwater designs were used in the analysis. The first had an 8-ft wide crest and the second a 45-ft wide crest. Both breakwater crests were at an elevation of +26 MLLW. Side slopes for both intercepted the mud line at an elevation of -25 MLLW and varied from 1½h:1v to 2h:1v. For purposes of the analysis, the water level was assumed to be -4 MLLW. See Figure 1.

Material Characteristics & Engineering Properties

A test pit reconnaissance exploration was performed at the Haines Small Boat Harbor in May 2001. The purpose of that exploration was to determine the general subsurface characteristics at the site and did not specifically address soil strength parameters. Consequently, assumed unit weight and shear strength values for foundation soils were established from engineering publications based on observations during the test pit exploration.

During the test pit exploration, the clay soils encountered were removed in large blocks taking the shape of the backhoe bucket. Using the standard field test of measuring clay soil strength by indenting a sample by thumb pressure, the geologist classified the clay's consistency as stiff. Published engineering data characterizes the undrained shear strength of a stiff clay from 1000 to 2000 pounds per square foot (psf). The lower shear strength of 1000 psf was used for this analysis.

Method of Analysis and Results

The Corp of Engineers' slope stability program UTEXAS 4 was used to perform the stability analysis. The results of the program were verified by hand calculations.

Since the clay soil is below the lowest tide level a saturated unit weight was assumed. In addition, its shear strength was assumed to be constant with depth. Typical values based on past experience was used for the armor rock. Seismic considerations were not addressed.

Using engineering publications and field observations during the test pit exploration, the following soil properties were assumed for the various construction materials and in-situ conditions:

Soil Description	Weight	Friction Angle	Cohesion
Unit weight of clay, saturated Unit weight of Saturated Armor Rock Unit weight of Emergent Armor Rock	115 pcf	0 deg.	1000 psf
	126 pcf	45 deg.	0 psf
	100 pcf	45 deg.	0 psf

The results of the analysis indicate the clay to be capable of supporting the proposed breakwaters as currently designed. The factors of safety range from 1.3 to 1.6. A factor of safety of 1.4 was computed for 1½h:1v side slopes and 1.6 for 2h:1v side slopes. A factor of safety of 1.3, independent of the two side slopes, was computed for a 45-ft breakwater crest. A minimum value of 1.3 is normally recommended for conditions where there is no threat to human life.

Figure 2 depicts the computed failure plane for the 1½h:1v condition. Note, as expected, the failure is a deep failure within the clay foundation.

Conclusion

On the basis of the information provided by H&H and the assumed strength parameters, the slope analysis indicates the foundation clay to be capable of supporting the proposed breakwaters.

Since this analysis uses assumed values for the shear strength and unit weight of the clay foundation soils, the results are to be used for preliminary design purposes and are subject to change with additional information. A final analysis will be performed upon completion of a planned future detailed geotechnical investigation of the site. That investigation will include sampling methods to more accurately determine unit weights and strength values of the foundation materials.

